

STEADY MOTION OF ICE SHEETS

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ABSTRACT. Steady plane flow under gravity of a symmetric ice sheet resting on a horizontal rigid bed, subject to surface accumulation and ablation, basal drainage, and basal sliding according to a shear-traction-velocity power law, is treated. The surface accumulation is taken to depend on height, and the drainage and sliding coefficient also depend on the height of overlying ice. The ice is described as a general non-linearly viscous incompressible fluid, with illustrations presented for Glen's power law, the polynomial law of Colbeck and Evans, and a Newtonian fluid. Uniform temperature is assumed so that effects of a realistic temperature distribution on the ice response are not taken into account. In dimensionless variables a small parameter ν occurs, but the $\nu = 0$ solution corresponds to an unbounded sheet of uniform depth. To obtain a bounded sheet, a horizontal coordinate scaling by a small factor $\epsilon(\nu)$ is required, so that the aspect ratio ϵ of a steady ice sheet is determined by the ice properties, accumulation magnitude, and the magnitude of the central thickness. A perturbation expansion in ϵ gives simple leading-order terms for the stress and velocity components, and generates a first order non-linear differential equation for the free-surface slope, which is then integrated to determine the profile. The non-linear differential equation can be solved explicitly for a linear sliding law in the Newtonian case. For the general law it is shown that the leading-order approximation is valid both at the margin and in the central zone provided that the power and coefficient in the sliding law satisfy certain restrictions.

RÉSUMÉ. *Mouvement des calottes glaciaires en écoulement permanent.* On traite de l'écoulement permanent plan sous l'effet de la gravité d'une calotte glaciaire symétrique reposant sur un lit horizontal rigide sous l'influence de l'accumulation et de l'ablation de surface avec évacuation au fond et glissement sur le lit selon une loi exponentielle vitesse/cisaillage. On admet que la surface d'accumulation dépend de la hauteur, les coefficients d'écoulement et de glissement dépendent aussi de la hauteur de la glace susjacente. La glace est considérée comme un fluide incompressible, à viscosité non linéaire, avec des exemples présentés suivant la loi exponentielle de Glen, la loi polynomiale de Colbeck et Evans et la loi de Newton. On admet une température uniforme de sorte que les effets de la distribution réelle des températures sur le comportement de la glace ne sont pas pris en compte. En variables sans dimensions un petit paramètre ν intervient, mais la solution $\nu=0$ correspond à une calotte non limitée d'épaisseur uniforme. Pour obtenir une calotte finie, il faut pondérer les coordonnées horizontales par un petit facteur $\epsilon(\nu)$ de sorte que le rapport de relief ϵ d'une calotte en état d'équilibre est déterminé par les propriétés de la glace, l'importance de l'accumulation et l'ordre de grandeur de l'épaisseur au centre. Une perturbation par expansion de ϵ donne des modifications simples des termes réglant les composantes de contrainte et de vitesse, et entraîne une équation différentielle non linéaire de premier ordre pour la pente de la surface libre, qu'on peut alors intégrer pour déterminer le profil. L'équation différentielle linéaire peut être résolue explicitement pour une loi linéaire de glissement dans le cas de l'écoulement Newtonien. Pour la loi générale, on montre que l'approximation de premier ordre est valable à la fois sur la zone de bordure et au centre pourvu que l'exposant et le coefficient de la loi de glissement satisfassent certaines conditions.

ZUSAMMENFASSUNG. *Stationäre Bewegung von Eisschilden.* Es wird der stationäre, ebene Fluss unter Schwerkraft eines symmetrischen Eisschildes behandelt, der auf einem horizontalen starren Bett auf liegt, an seiner Oberfläche Akkumulation und Ablation erfährt, ein Abflusssystem am Untergrund besitzt und dort nach einem Exponentialgesetz zwischen Scherspannung und Geschwindigkeit gleitet. Für die Akkumulation an der Oberfläche wird Höhenabhängigkeit angenommen; auch der Abfluss und der Gleitkoeffizient sollen von der Höhe des überlagernden Eises abhängen. Das Eis gilt als eine im allgemeinen nicht linear viskose, inkompressible Flüssigkeit, wobei Beispiele für Glen's Exponentialgesetz, für das Polynomgesetz von Colbeck und Evans und für eine Newton'sche Flüssigkeit herangezogen werden. Es wird gleichförmige Temperatur angenommen, weshalb die Einflüsse einer tatsächlichen Temperaturverteilung auf das Verhalten des Eises nicht berücksichtigt werden. Bei dimensionslosen Variablen tritt ein kleiner Parameter ν auf, doch entspricht die Lösung für $\nu=0$ einem unbegrenzten Eisschild konstanter Dicke. Für einen begrenzten Eisschild wird eine horizontale Koordinatenbeziehung mit einem kleinen Faktor $\epsilon(\nu)$ benötigt, so dass das Verhältnis ϵ eines stationären Eisschildes durch die Eigenschaften des Eises, die Größe der Akkumulation und die Dicke im Zentrum bestimmt wird. Eine Störungsexpansion in ϵ gibt einfache Richtwertausdrücke für die Komponenten der Spannung und Geschwindigkeit und führt zu einer nichtlinearen Differentialgleichung 1. Ordnung für die Neigung der freien Oberfläche, deren Integration das Profil liefert; sie kann explizit für ein lineares Gleitgesetz im Newton'schen Fall gelöst werden. Für den allgemeinen Fall wird gezeigt, dass die Richtwertnäherung sowohl am Rand wie im Zentrum gilt, sofern die Potenz und der Koeffizient im Gleitgesetz bestimmten Einschränkungen genügen.

1. INTRODUCTION

The first theoretical treatment of a bounded ice sheet with steady free surface, a property of the flow solution, was given by Nye (1959). His basic problem was a two-dimensional symmetric ice sheet on a horizontal bed, in steady plane flow under gravity, with a steady

profile maintained by surface accumulation and ablation. This is illustrated by the cross-section in Figure 1 with horizontal bed $y = 0$ and surface profile $y = h(x)$ in rectangular Cartesian coordinates $Oxyz$, and all physical variables are independent of z . The velocity components are $(u, v, 0)$, g is the acceleration due to gravity acting in the negative y -direction, σ is the Cauchy stress tensor with $\sigma_{xz} = \sigma_{yz} = 0$, and (t_s, t_n) denote tangential and normal tractions on the surface.

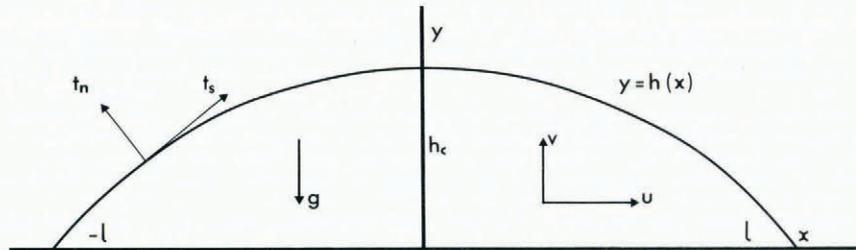


Fig. 1. Ice-sheet cross-section.

Nye notes that in the ice sheets of Antarctica and Greenland the relative velocity between surface and bed is small compared with basal sliding, and, while the differential motion in glaciers is not negligible, it is largely concentrated in a thin basal shear layer. Using an estimated temperature profile with depth and a temperature-dependent flow law, Nye calculates the velocity and stress profiles under a point on the Greenland ice sheet, and concludes that any significant shear motion must be concentrated in the warmest layer at the bottom. He therefore proposes the approximation that the horizontal velocity is uniform with depth and is determined through a sliding law by the basal shear stress. In particular he makes the assumption

$$u = u(x) = \pm \{ \pm \sigma_{xy}(y = 0) / A \}^m, \quad x \geq 0. \quad (\text{N1})$$

A and m are constants, which will depend on bed roughness, temperature, and basal ice structure; $m = 2.5$ is used for calculations. If a concentrated shear layer occurs near the bed, then (N1) applies at the base of the mean uniform velocity region. A rigorous derivation of a sliding law such as (N1) to describe mean flow conditions over a smoothed bed contour has yet to be found.

While Nye's subsequent analysis is independent of the flow law and any temperature dependence, he has, nevertheless, deduced that observed temperature profiles give rise to an approximately uniform horizontal velocity profile above a thin bottom layer. In this paper we determine the horizontal velocity distribution by considering the full field equations, but on the assumption of uniform temperature so that Nye's deduction can not be tested, nor can we predict the effect of temperature variation. However, our solutions demonstrate the errors produced by the same approximate procedure applied to the uniform temperature case, but more important is the presentation of a rational approximation scheme which appears to be appropriate to related free-surface problems involving other features, including temperature effects.

Nye next assumes that the surface slope is small, $|h'(x)| \ll 1$, and approximates the basal shear stress by

$$\sigma_{xy}(y = 0) = -\rho g h h', \quad (\text{N2})$$

where ρ is the uniform ice density. Equation (N2) is based on an equilibrium argument (Nye, 1952), which supposes that the stress gradients parallel to the surface are negligible, and also

implies $\sigma_{xx} \approx \sigma_{yy}$. Eliminating σ_{xy} between Equations (N1) and (N2), and expressing uh as the integrated surface accumulation by the assumption of incompressibility, gives a non-linear first-order differential equation for $h(x)$. For uniform accumulation q , Nye (1959) obtains the solution

$$\left(\frac{h}{h_c}\right)^{2+1/m} + \left(\frac{|x|}{l}\right)^{1+1/m} = 1, \tag{N3}$$

where

$$\rho g(m+1) h_c^{2+1/m} = Aq^{1/m}(2m+1) l^{1+1/m}. \tag{N4}$$

Thus, for given A , m , and q , the centre height h_c is related to the semi-length l , and either h_c , or l , or the aspect ratio h_c/l , must be specified to complete the solution. The assumption of constant q is inconsistent with zero net mass flux across the surface required for a steady profile, but Equation (N3) fails as $|x| \rightarrow l$ where the slope $|h'|$ becomes infinite, and violates the basic assumption when $|h'|$ is no longer small compared with unity, so there is an unspecified margin range with unknown profile where the necessary ablation takes place. Weertman (1961) also suggests that a requirement of slowly varying slope is implicit in Nye's arguments, and is not satisfied by Equation (N3) in some central region. He further proposes that a non-zero effective longitudinal stress $\sigma_{xx} - \sigma_{yy}$ can be accounted for by using a stress invariant in the boundary condition (N1), by analogy with Glen's three-dimensional constitutive law, but retains Equation (N2) and gives no further consideration to momentum or constitutive equations.

We have therefore re-examined the steady-motion-steady-profile problem shown in Figure 1, treating the ice as a general non-linearly viscous fluid, prescribing the accumulation (ablation) as a function of the height of the free surface, prescribing basal drainage as a function of the overlying height of ice, and adopting a basal sliding law of the form of Equation (N1) but with a coefficient $\lambda(h)$ depending on the overlying height of ice. Thus Equation (N1) is replaced by

$$y = 0: \quad \sigma_{xy} = \pm \lambda(h)[\pm u]^{1/m}, \quad x \gtrless 0. \tag{1}$$

Note that Equation (1) assumes that basal shear stress and velocity are everywhere directed away from the centre. Basal drainage is described by

$$y = 0: \quad v = -b(h) \leq 0. \tag{2}$$

If q is the accumulation (volume flux per unit area) per unit horizontal cross-section, and q_n that per unit surface area, then

$$y = h(x): \quad v - h'u = -q(h) = -q_n(h)[1+h'^2]^{\frac{1}{2}}. \tag{3}$$

The normal form may be more appropriate on steep slopes, particularly for ablation zones $q < 0$. Assuming that atmospheric pressure is uniform over the surface, and measuring stress relative to this isotropic pressure, the free-surface conditions are

$$2(1+h'^2) t_n = (1+h'^2)(\sigma_{xx} + \sigma_{yy}) + (1-h'^2)(\sigma_{yy} - \sigma_{xx}) - 4h'\sigma_{xy} = 0, \tag{4}$$

$$(1+h'^2) t_s = h'(\sigma_{yy} - \sigma_{xx}) + (1-h'^2) \sigma_{xy} = 0. \tag{5}$$

The problem may be solved for $x < 0$ or for $x > 0$ by adjoining the symmetry conditions

$$x = 0: \quad u = 0, \quad \sigma_{xy} = 0, \quad h' = 0. \tag{6}$$

Formulation of the full problem in dimensionless variables introduces a parameter ν , for which only a magnitude of h_c , not h_c itself, is prescribed in the definition. For a wide range of practical conditions $\nu \ll 1$. The approximation $\nu = 0$ gives a solution only for an unbounded sheet of uniform depth, not permitting a margin at finite distance l from the centre. This may be treated as an approximate solution valid in some central region—an "outer

solution³ in a matching procedure—but an intermediate scaling of the horizontal coordinate by a factor $\epsilon(\nu) \ll 1$ determines an approximate solution valid also in the central region. The parameter ϵ defines the small surface-slope magnitude, and is determined by the ice properties, accumulation magnitude, and magnitude of h_c . Further, this small-slope solution is valid up to the margin ($h = 0$) provided that the sliding coefficient $\lambda(h) \simeq \lambda_0 h$ as $h \rightarrow 0$. Here we present the leading-order small-slope solution under this validity condition. The central height h_c and semi-length l (and hence the aspect ratio, h_c/l) are determined by the solution, in contrast to Equations (N3) and (N4) where one parameter must be prescribed. From this leading-order solution Nye's assumptions

$$|\sigma_{xx} - \sigma_{yy}| \ll |\sigma_{xy}| \ll |\sigma_{xx} + \sigma_{yy}|, \quad (7)$$

and Equation (N2) are confirmed, but the profile, and consequently the shear stress and velocity distributions, are quite different from those of Nye (1959) and Weertman (1961).

Illustrations are presented for Glen's power law, the polynomial law of Colbeck and Evans, and a Newtonian fluid. The infinite viscosity at zero stress implied by the power law requires a more elaborate scaling procedure, but the analysis and solution are presented since the power law has become the conventional model. However, this model was introduced to describe overall response over a wide stress range, and not an accurate response near zero stress, and response functions which exhibit finite viscosity at zero stress are probably a better physical description as well as leading to more straightforward analysis.

2. BALANCE LAWS AND CONSTITUTIVE LAWS

Mass balance for an incompressible material implies

$$\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} = 0. \quad (8)$$

Inertia terms are negligible in this slow viscous flow so momentum balance requires

$$\frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xy}}{\partial y} = 0, \quad \frac{\partial \sigma_{xy}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} - \rho g = 0. \quad (9)$$

The ice is assumed to be an incompressible non-linearly viscous fluid with a temperature-dependent rate factor (Morland, 1979). A convenient representation for use in Equation (9) is

$$\frac{\boldsymbol{\sigma} + p\mathbf{I}}{\sigma_0} = \phi_1(I_2, I_3) \hat{\mathbf{D}} + \phi_2(I_2, I_3) [\hat{\mathbf{D}}^2 - \frac{2}{3} I_3 \mathbf{I}], \quad (10)$$

where

$$p = -\frac{1}{3} \text{tr } \boldsymbol{\sigma}, \quad I_2 = \frac{1}{2} \text{tr } \hat{\mathbf{D}}^2, \quad I_3 = \det \hat{\mathbf{D}}, \quad (11)$$

and

$$D_{ij} = \frac{1}{2} \left(\frac{\partial v_i}{\partial x_j} + \frac{\partial v_j}{\partial x_i} \right), \quad \hat{\mathbf{D}} = \mathbf{D}/D_0 a(T). \quad (12)$$

T denotes temperature and $a(T)$ is the rate factor, normalized by $a(T_0) = 1$ for some temperature T_0 , with $a'(T) \geq 0$. σ_0 and D_0 denote a constant stress magnitude and a constant strain-rate magnitude respectively, so that $\hat{\mathbf{D}}$, the invariants I_2, I_3 , and response functions ϕ_1, ϕ_2 are dimensionless. The alternative general representation for strain-rate in terms of deviatoric stress, given by Glen (1958), cannot be used to eliminate stress from Equations (9).

However, the laws commonly adopted for \mathbf{D} give the simpler form

$$\hat{\mathbf{D}} = \omega(\mathcal{J}_2) \frac{\boldsymbol{\sigma} + p\mathbf{I}}{\sigma_0}, \quad \mathcal{J}_2 = \frac{1}{2} \text{tr} \left(\frac{\boldsymbol{\sigma} + p\mathbf{I}}{\sigma_0} \right)^2, \tag{13}$$

which implies

$$\left. \begin{aligned} \hat{I}_2 &= \mathcal{J}_2 \omega^2(\mathcal{J}_2) = G(\mathcal{J}_2) \text{ say,} & \phi_2 &\equiv 0, \\ \phi_1 &= \phi_1(\hat{I}_2), & \mathcal{J}_2 &= \hat{I}_2 \phi_1^2(\hat{I}_2) = G^{-1}(\hat{I}_2). \end{aligned} \right\} \tag{14}$$

For example, with

$$\sigma_0 = 10^5 \text{ N m}^{-2}, \quad D_0^{-1} = 3 \times 10^7 \text{ s}, \quad T_0 = 273 \text{ K}, \tag{15}$$

Glen's law deduced from uniaxial compression data is represented by

$$\left. \begin{aligned} \omega(\mathcal{J}_2) &= 1.5k(3\mathcal{J}_2)^{(n-1)/2}, & \phi_1(\hat{I}_2) &= \frac{2}{3} k^{-1/n} \left(\frac{4\hat{I}_2}{3} \right)^{-(n-1)/2n}, \\ k &= 0.17, & n &= 1.3 \rightarrow 4, \end{aligned} \right\} \tag{16}$$

n depending on the stress range over which the response is approximated by this law. The singularity in $\phi_1(\hat{I}_2)$ as $\hat{I}_2 \rightarrow 0$ ($\mathcal{J}_2 \rightarrow 0$) implies an infinite viscosity at zero stress, and complicates the order of magnitude analysis of the problem described in Section 1. However, the analysis for such a singularity is presented since Glen's law is the conventional model. More accurate data fitting over a low stress range suggests a finite viscosity at zero stress, and for $0 \rightarrow 10^5 \text{ N m}^{-2}$ Colbeck and Evans (1973) propose a polynomial law represented by

$$\left. \begin{aligned} \omega(\mathcal{J}_2) &= 1.5(C_0 + 3C_1\mathcal{J}_2 + 9C_2\mathcal{J}_2^2), \\ C_0 &= 0.21, \quad C_1 = 0.14, \quad C_2 = 0.055. \end{aligned} \right\} \tag{17}$$

$\phi(\hat{I}_2)$ cannot be obtained by analytic inversion of Equations (14), but our leading-order solution can use Equations (16) or (17) directly.

From Equations (13) and (17) the viscosities at $\mathcal{J}_2 = 0$ and $\mathcal{J}_2 = 1$ (a deviatoric stress of 1 bar) are

$$\mu(0) \approx \frac{5 \times 10^{12}}{a(T)} \text{ N m}^{-2} \text{ s}, \quad \mu(1) \approx \frac{10^{12}}{a(T)} \text{ N m}^{-2} \text{ s}, \tag{18}$$

and the mean viscosity commonly assumed for a Newtonian model near pressure melting is $\mu \approx 3 \times 10^{12} \text{ N m}^{-2} \text{ s}$. As T decreases from T_0 , $a(T)$ decreases from unity and the viscosity at each stress increases; at $T = 250 \text{ K}$ the viscosity is increased by a factor ten (Paterson, 1969, p. 83), so $a(250 \text{ K}) \approx 0.1$. Increases are less pronounced for further decreases of temperature. In the subsequent analysis we assume a uniform temperature and take $a=0.1$ for magnitude estimates. For a non-uniform temperature field the factor $a[T(x)]$ introduces non-homogeneous response. In adopting the general law (10) we assume that the stress contribution from a non-zero ϕ_2 term is not of greater magnitude than that of the ϕ_1 term. Also it is assumed that ϕ_2 is bounded, so $|\phi_2| \leq O(1)$, and ϕ_1 can have a singularity only of the form given in Equations (16).

3. DIMENSIONLESS FORMULATION AND THE SMALL PARAMETER

We introduce dimensionless variables by

$$\left. \begin{aligned} (x, y, h, l) &= h_0(X, Y, H, L), & (\boldsymbol{\sigma}, p) &= \rho g h_0(\boldsymbol{\Sigma}, P), \\ (u, v, q, b) &= q_m(U, V, Q, B), \end{aligned} \right\} \tag{19}$$

where h_0 is a magnitude of the maximum ice thickness ($h_c = h_0 H_c$ is determined by the solution,

with H_c order unity), and q_m is a magnitude of maximum accumulation density, so P and Q are of order unity. We will suppose $q_m = q(h_0)$ on the assumption that ablation at lower heights does not significantly exceed this value; otherwise the ablation maximum should be used. We suppose that the drainage B is not greater in magnitude than Q . We can then define

$$h'(x) = H'(X) \equiv \Gamma(H). \quad (20)$$

Setting

$$U = \frac{\partial \Psi}{\partial X}, \quad V = -\frac{\partial \Psi}{\partial Y}, \quad (21)$$

in terms of a dimensionless stream function $\Psi(X, Y)$, satisfies the mass balance Equation (8). The momentum equations (9) become

$$\frac{\partial \Sigma_{xx}}{\partial X} + \frac{\partial \Sigma_{xy}}{\partial Y} = 0, \quad \frac{\partial \Sigma_{xy}}{\partial X} + \frac{\partial \Sigma_{yy}}{\partial Y} = 1. \quad (22)$$

Boundary conditions (1)–(6) become, for $Y = 0$:

$$\Sigma_{xy} = \pm \Lambda(H) \left(\pm \frac{\partial \Psi}{\partial Y} \right)^{1/m}, \quad X \geq 0, \quad \Lambda = \frac{\lambda q_m^{1/m}}{\rho g h_0}, \quad (23)$$

$$\frac{\partial \Psi}{\partial X} = B(H), \quad (24)$$

for $Y = H(X)$:

$$\frac{\partial \Psi}{\partial X} + \Gamma(H) \frac{\partial \Psi}{\partial Y} = Q(H) = Q_n(H) [1 + \Gamma^2(H)]^{1/2}, \quad (25)$$

$$[1 + \Gamma^2(H)] (\Sigma_{xx} + \Sigma_{yy}) + [1 - \Gamma^2(H)] (\Sigma_{yy} - \Sigma_{xx}) - 4\Gamma(H) \Sigma_{xy} = 0, \quad (26)$$

$$\Gamma(H) [\Sigma_{yy} - \Sigma_{xx}] + [1 - \Gamma^2(H)] \Sigma_{xy} = 0, \quad (27)$$

and for $X = 0$:

$$\frac{\partial \Psi}{\partial Y} = 0, \quad \Sigma_{xy} = 0, \quad \Gamma = 0. \quad (28)$$

The tensors $\hat{\mathbf{D}}, \hat{\mathbf{D}}^2$ have non-zero components

$$\left. \begin{aligned} \hat{D}_{xx} = -\hat{D}_{yy} &= \delta \frac{\partial^2 \Psi}{\partial X \partial Y}, & \hat{D}_{xy} &= \frac{1}{2} \delta \left(\frac{\partial^2 \Psi}{\partial Y^2} - \frac{\partial^2 \Psi}{\partial X^2} \right), \\ (\hat{\mathbf{D}}^2)_{xx} = (\hat{\mathbf{D}}^2)_{yy} &= \hat{I}_2 = \delta^2 I_2, & \delta &= q_m / a D_0 h_0, \\ \hat{I}_3 &= 0, & I_2 &= \left(\frac{\partial^2 \Psi}{\partial X \partial Y} \right)^2 + \frac{1}{4} \left(\frac{\partial^2 \Psi}{\partial Y^2} - \frac{\partial^2 \Psi}{\partial X^2} \right)^2 \end{aligned} \right\} \quad (29)$$

Taking a central accumulation $q_m \approx 3 \times 10^{-9} \text{ m s}^{-1}$ (used by Nye (1959) with $h_0 \approx 3000 \text{ m}$), a rate factor $a = 0.1$, and the other variables taking the values given in Equations (15), gives

$$\left. \begin{aligned} h_0 = 3000 \text{ m}: & \quad \delta \approx 3 \times 10^{-4}, \\ h_0 = 100 \text{ m}: & \quad \delta \approx 10^{-2}, \end{aligned} \right\} \ll 1 \quad (30)$$

for a thick ice sheet and moderate glacier. In fact, with larger a and smaller q_m for a thin glacier, δ will be smaller than the larger value given in Equations (30), and the strong inequality $\delta \ll 1$ will apply in most, if not all, practical situations.

In Glen's law (16), ϕ_1 is unbounded as $l_2 \rightarrow 0$ ($n > 1$), but $\phi_1 \mathbf{D} \rightarrow 0$. For any singular ϕ_1 of this form, we can write

$$\phi_1 \mathbf{D} = \bar{\phi}_1 (\mathbf{D} l_2^{-\alpha}), \quad 0 \leq \alpha < \frac{1}{2}, \tag{31}$$

where $\mathbf{D} l_2^{-\alpha} \rightarrow 0$ as $l_2 \rightarrow 0$ and

$$\bar{\phi}_1 = l_2^\alpha \phi_1, \tag{32}$$

is bounded. For Glen's law

$$\alpha = \frac{n-1}{2n} < \frac{1}{2}, \quad \bar{\phi}_1 = \frac{2}{3} \left(\frac{3}{4}\right)^{(n-1)/2n} k^{-1/n} = O(1), \tag{33}$$

and $\bar{\phi}_1$ is constant, while the finite viscosity case is obtained by setting $\alpha = 0$, with $\phi_1 = O(1)$ by Equations (17). Hence Equations (31) and (32) cover both cases. From Equations (10) and (29),

$$\left. \begin{aligned} \left(\begin{matrix} \Sigma_{xx} \\ \Sigma_{yy} \end{matrix} \right) &= -P + \nu \left\{ \pm \bar{\phi}_1 l_2^{-\alpha} \frac{\partial^2 \Psi'}{\partial X \partial Y} + \frac{1}{3} \delta^{1+2\alpha} \phi_2 l_2 \right\}, \\ \Sigma_{xy} &= \frac{1}{2} \nu \bar{\phi}_1 l_2^{-\alpha} \left(\frac{\partial^2 \Psi'}{\partial Y^2} - \frac{\partial^2 \Psi'}{\partial X^2} \right), \end{aligned} \right\} \tag{34}$$

where

$$\nu = s \delta^{1-2\alpha}, \quad s = \frac{\sigma_0}{\rho g h_0}. \tag{35}$$

TABLE I. MAGNITUDES OF ν AND ϵ FOR A FINITE VISCOSITY LAW ($n = 1$) AND GLEN'S LAW WITH EXPONENT n

n h_0 m	1		2		3		4	
	ν	ϵ	ν	ϵ	ν	ϵ	ν	ϵ
3 000	10^{-6}	10^{-3}	6×10^{-5}	1.5×10^{-3}	2×10^{-4}	1.7×10^{-3}	4×10^{-4}	1.9×10^{-3}
100	10^{-3}	3×10^{-2}	10^{-2}	4.6×10^{-2}	2×10^{-2}	5×10^{-2}	3×10^{-2}	6×10^{-2}

For Glen's law, Equations (33), with $n = 1$ corresponding to the finite viscosity case, and the parameter values given by Equations (15) and (30), the thick ice sheet and moderate glacier values of ν are shown in Table I. Thus, recalling that a lower value of δ is expected for $h_0 = 100$ m, and that $n = 1$ is the most realistic law, Table I implies

$$\nu \ll 1, \tag{36}$$

which is the condition we adopt. Note also that

$$\theta = \delta/s = 0.09, \tag{37}$$

is independent of h_0 , and decreases with decrease of q_m and increase of a . For practical conditions we can assume $\theta \leq O(1)$. Assuming that P , Ψ , and their gradients, are not greater than order unity, the leading order of a series solution in ν is given by setting $\nu = 0$. In particular, from Equations (22) and (26),

$$P = H - Y, \quad H = \text{constant}, \quad \Gamma = 0, \tag{38}$$

which correspond to an unbounded sheet of uniform depth. That is, a margin at finite distance L cannot be obtained. Such a solution may represent an "outer" solution, valid in some central zone, to match with a solution which incorporates a surface slope, but as we now show that the indicated intermediate coordinate scaling leads to a solution uniformly valid to the centre, under a weak restriction on the sliding law, the ν expansion is not pursued.

4. SCALED VARIABLES AND SLOPE MAGNITUDE

For a non-zero surface slope, P , to leading order, must not be independent of X , so the horizontal momentum balance given by the first of Equations (22) must involve the shear-stress gradient in \mathcal{Y} . This suggests a horizontal coordinate contraction

$$\xi = \epsilon X, \quad \Gamma = \epsilon \gamma = \epsilon dH/d\xi, \tag{39}$$

where $\xi, \gamma = O(1)$, and the slope Γ has magnitude ϵ . To retain the surface accumulation balance given by Equation (25), we need also a stream-function scaling

$$\psi = \epsilon \Psi = O(1), \tag{40}$$

so Equation (25) becomes

$$\mathcal{Y} = H: \quad \frac{\partial \psi}{\partial \xi} + \gamma \frac{\partial \psi}{\partial \Gamma} = Q(H). \tag{41}$$

Now, by Equations (29) and (34),

$$I_2 = \epsilon^{-2}i, \quad i = \frac{1}{4} \left(\frac{\partial^2 \psi}{\partial \Gamma^2} \right)^2 + \epsilon^2 \left[\left(\frac{\partial^2 \psi}{\partial \xi \partial \Gamma} \right)^2 - \frac{1}{2} \frac{\partial^2 \psi}{\partial \xi^2} \frac{\partial^2 \psi}{\partial \Gamma^2} \right] + \frac{1}{4} \epsilon^4 \left(\frac{\partial^2 \psi}{\partial \xi^2} \right)^2 = O(1), \tag{42}$$

$$\left. \begin{aligned} \left(\begin{matrix} \Sigma_{xx} \\ \Sigma_{yy} \end{matrix} \right) &= -P + \nu \left\{ \pm \epsilon^{2\alpha} \phi_1 i^{-\alpha} \frac{\partial^2 \psi}{\partial \xi \partial \Gamma} + \frac{1}{3} \epsilon^{-2} \delta^{1+2\alpha} \phi_2 i \right\}, \\ \Sigma_{xy} &= \frac{1}{2} \nu \epsilon^{2\alpha-1} \phi_1 i^{-\alpha} \left(\frac{\partial^2 \psi}{\partial \Gamma^2} - \epsilon^2 \frac{\partial^2 \psi}{\partial \xi^2} \right). \end{aligned} \right\} \tag{43}$$

The terms of the first of Equations (22) now have orders of magnitude $\epsilon, \nu \epsilon^{2\alpha+1}, \nu \epsilon^{-1} \delta^{2\alpha+1}, \nu \epsilon^{2\alpha-1}, \nu \epsilon^{2\alpha+1}$, and a balance for the P -gradient with the shear-stress gradient requires

$$\nu \epsilon^{2\alpha-2} = O(1), \quad \nu \epsilon^{-2} \delta^{2\alpha+1} \leq O(1). \tag{44}$$

To satisfy the equality, take

$$\left. \begin{aligned} \epsilon &= \nu^{1/(2-2\alpha)} = \nu^{n/n+1} = \delta \theta^{-n/(n+1)}, \\ \frac{\epsilon}{\epsilon(n=1)} &= \theta^{-(n-1)/2(n+1)} \approx \left\{ \begin{array}{l} 2 \text{ for } \theta = 0.09 \\ 8 \text{ for } \theta = 10^{-3} \\ 16 \text{ for } \theta = 10^{-4} \end{array} \right\} \text{ all for } n = 4. \end{aligned} \right\} \tag{45}$$

Then the term in the inequality in (44) is $\epsilon \theta^{(2\alpha+1)/(2-2\alpha)} \leq O(\epsilon)$. Thus the ϕ_2 term does not contribute to Σ_{xx}, Σ_{yy} to leading order with the assumption $|\phi_2| \leq O(1)$. Equations (45) show values of the ratio of the coordinate scale factor between $n = 1$ and $n = 4$, the extreme range, for a wide range of θ . Clearly the ratio is of order unity when θ is not too small or when n is close to unity. Table I shows the magnitudes of ϵ for the large and small h_0 and various values of the exponent n . Again, $n = 1$ is the most realistic law, and smaller ν is expected for $h_0 = 100$ m, so in most, if not all, practical situations,

$$\epsilon \ll 1. \tag{46}$$

From Equations (29), (42), and (45), we have the identity

$$i = \theta^{-1/(1-\alpha)} \hat{I}_2. \tag{47}$$

5. LEADING-ORDER APPROXIMATION

We now seek the leading-order approximation of a series expansions in ϵ . Let $P = P_0 + o(1), \psi = \psi_0 + o(1)$, and

$$H = \eta(\xi) + o(1), \quad \gamma_0 = \eta'(\xi). \tag{48}$$

First derivatives of P_0 and first to third derivatives of ψ_0 arise in the momentum balance, so these derivatives must be of order unity or less in a valid approximation. This is confirmed under weak restrictions on the sliding-law parameters. Denoting all leading-order quantities by a subscript or superscript 0, we have

$$\left. \begin{aligned} \Sigma_{xx}^0 &= \Sigma_{yy}^0 = -P_0, \\ \Sigma_{xy}^0 &= \frac{1}{2} \epsilon \bar{\phi}_1 i_0^{-\alpha} \frac{\partial^2 \psi_0}{\partial Y^2}, \quad i_0 = \left(\frac{1}{2} \frac{\partial^2 \psi_0}{\partial Y^2} \right)^2, \\ (\Sigma_{xx} - \Sigma_{yy})^0 &= 2\epsilon^2 \bar{\phi}_1 i_0^{-\alpha} \frac{\partial^2 \psi_0}{\partial \xi \partial Y}, \\ U_0 &= \epsilon^{-1} \frac{\partial \psi_0}{\partial Y}, \quad V_0 = -\frac{\partial \psi_0}{\partial \xi}, \end{aligned} \right\} \quad (49)$$

which confirm the magnitude relations (7). Recall that $\bar{\phi}_1$ is evaluated at $\bar{I}_2 = \theta^{1/(1-\alpha)} i_0$, $\bar{I}_3 = 0$.

The momentum balance equations (22) become

$$\left. \begin{aligned} -\frac{\partial P_0}{\partial \xi} + \frac{\partial}{\partial Y} \left\{ \bar{\phi}_1 \left(\frac{1}{2} \frac{\partial^2 \psi_0}{\partial Y^2} \right) \left[\left(\frac{1}{2} \frac{\partial^2 \psi_0}{\partial Y^2} \right)^2 \right]^{-\alpha} \right\} &= 0, \\ -\frac{\partial P_0}{\partial Y} &= 1. \end{aligned} \right\} \quad (50)$$

On $Y = 0$, by Equations (23) and (24),

$$\left. \begin{aligned} \left(\frac{1}{2} \frac{\partial^2 \psi_0}{\partial Y^2} \right) \left[\left(\frac{1}{2} \frac{\partial^2 \psi_0}{\partial Y^2} \right)^2 \right]^{-\alpha} \bar{\phi}_1 &= \pm j \bar{\Lambda}(\eta) \left(\pm \frac{\partial \psi_0}{\partial Y} \right)^{1/m}, \quad \xi \geq 0, \\ \frac{\partial \psi_0}{\partial \xi} &= B(\eta), \end{aligned} \right\} \quad (51)$$

where

$$i = \theta^{(n-1)(m+1)/2m(n+1)} \leq O(1), \quad \bar{\Lambda} = \frac{\lambda q m^{1/m}}{\rho g h_0} \left(\frac{\theta}{\delta^2} \right)^{(m+1)/2m}; \quad (52)$$

$\bar{\Lambda}$ is independent of n . This definition of $\bar{\Lambda}$ is normalized on the coordinate scale $\epsilon_1 = \epsilon(n = 1)$ to be independent of the exponent n used, and j is order unity except for $n > 3$ or $\theta < 10^{-2}$; $i = 1$ for $n = 1$. We suppose that the sliding law influences the leading-order solution, implicit in Nye's (1959) assumptions, so regard $j\bar{\Lambda}$ as order unity in the analysis. Perfect slip is given by $\bar{\Lambda} = 0$, and non-slip by $\bar{\Lambda} \rightarrow \infty$. On $\xi = 0$, by Equation (28):

$$\frac{\partial \psi_0}{\partial Y} = 0, \quad \frac{\partial^2 \psi_0}{\partial Y^2} = 0, \quad \gamma_0 = 0. \quad (53)$$

On $Y = \eta(\xi)$, by Equations (26), (27), (49), and (41):

$$\left. \begin{aligned} P_0 &= 0, \quad \frac{\partial^2 \psi_0}{\partial Y^2} = 0, \\ \frac{\partial \psi_0}{\partial \xi} + \gamma_0(\eta) \frac{\partial \psi_0}{\partial Y} &= Q(\eta). \end{aligned} \right\} \quad (54)$$

By the second of Equations (50) and the first of Equations (54),

$$P_0 = \eta(\xi) - Y. \quad (55)$$

Hence, by the first of Equations (50) and the second of Equations (54),

$$-\eta'(\xi)[\eta(\xi) - Y] = \pm \phi_1 i_0^{1-\alpha} = \pm \theta^{-(1-2\alpha)/(2-2\alpha)} (\hat{I}_2^0)^{\frac{1}{2}} \phi_1, \quad \frac{\partial^2 \psi_0}{\partial Y^2} \geq 0, \quad (56)$$

using Equations (32), (42), and (47). Thus the second of Equations (49) gives

$$\Sigma_{xy}^0 = -\epsilon \eta'(\xi)[\eta(\xi) - Y], \quad (57)$$

which confirms Equation (N2). Now by Equations (49), Σ_{xy}^0 and $\partial^2 \psi_0 / \partial Y^2$ have the same sign, and Equation (56) shows that this sign is unchanged at constant ξ , being the opposite sign to $\eta'(\xi)$. In turn, Equation (51) shows that $U_0(\xi, 0)$ has the opposite sign to $\eta'(\xi)$. Thus, for the sliding law given by Equation (1) with basal velocity directed away from the centre,

$$\eta'(\xi) \leq 0 \quad \text{for } \xi \geq 0, \quad (58)$$

which imply monotonic profiles in $\xi > 0$ and $\xi < 0$ separately. Alternatively, monotonic profiles imply an unchanged sliding direction.

For laws of the form of Equations (13), using the relations (14),

$$\left(\frac{1}{2} \frac{\partial^2 \psi_0}{\partial Y^2} \right)^2 = \theta^{-1/(1-\alpha)} G \{ \theta^{(1-2\alpha)/(1-\alpha)} [\eta'(\xi)]^2 [\eta(\xi) - Y]^2 \}, \quad (59)$$

which does not require explicit inversion of Equations (13). Given $\phi_1(\hat{I}_2, 0)$ in the general law given by Equation (10), Equation (56) must be solved for \hat{I}_2 to obtain the form of Equation (59). For Glen's law (16), Equation (59) becomes

$$\frac{\partial^2 \psi_0}{\partial Y^2} = \pm 3^{\frac{1}{2}(n+1)} k [\mp \eta'(\eta - Y)]^n, \quad (60)$$

and for the polynomial law (17), for which $\alpha = 0$,

$$\frac{\partial^2 \psi_0}{\partial Y^2} = -3\eta'(\eta - Y) \{ C_0 + 3C_1 \theta (\eta')^2 (\eta - Y)^2 + 9C_2 \theta^2 (\eta')^4 (\eta - Y)^4 \}. \quad (61)$$

A Newtonian fluid approximation with constant viscosity μ is given by Equation (60) or (61) with

$$n = 1, \quad C_1 = C_2 = 0, \quad C_0 = k = \frac{\sigma_0}{3aD_0\mu}. \quad (62)$$

For example, taking $a\mu = 3 \times 10^{12} \text{ N m}^{-2}$, based on Nye's temperate viscosity, gives $C_0 = k = 0.33$. In Equation (61), if $\theta \ll 1$, the higher-power terms make little contribution, which implies that the deviatoric stress is much smaller than σ_0 ; that is $|\Sigma_{xy}|, |\Sigma_{xx} - \Sigma_{yy}| \ll s$. By Equations (49) and (45), $|\Sigma_{xy}|/s = O(\theta^{(1-2\alpha)/(2-2\alpha)})$, so the shear stress only approaches 1 bar if $\theta \approx 1$; that is, $q_m \sim 3a \times 10^{-7} \text{ m s}^{-1}$.

From Equation (59),

$$\frac{\partial^2 \psi_0}{\partial Y^2} = \pm g[\mp \eta'(\xi)\{\eta(\xi) - Y\}], \quad (63)$$

with $g(t)$ given by Equation (60) for the power law and by Equation (61) for the polynomial law. Define

$$g_1(t) = \int_0^t g(t') dt', \quad g_2(t) = \int_0^t g_1(t') dt', \quad (64)$$

then integrating Equation (63):

$$\left. \begin{aligned} \frac{\partial \psi_0}{\partial Y} &= \frac{1}{\eta'} g_1[\mp \eta'(\eta - Y)] + f_1(\xi), \\ \psi_0 &= \pm \frac{1}{(\eta')^2} g_2[\mp \eta'(\eta - Y)] + Y f_1(\xi) + f_0(\xi), \\ \frac{\partial \psi_0}{\partial \xi} &= \pm \frac{\partial}{\partial \xi} \left\{ \frac{g_2[\mp \eta'(\eta - Y)]}{(\eta')^2} \right\} + Y f_1'(\xi) + f_0'(\xi). \end{aligned} \right\} \quad (65)$$

Hence, by the second of Equations (51)

$$f_0'(\xi) = B(\eta) \mp \frac{d}{d\xi} \left\{ \frac{g_2[\mp \eta \eta']}{(\eta')^2} \right\}, \quad (66)$$

and by the first of Equations (51) and Equation (56)

$$f_1(\xi) = \pm \left[\frac{\mp \eta \eta'}{j \bar{\Lambda}(\eta)} \right]^m - \frac{g_1(\mp \eta \eta')}{\eta'}. \quad (67)$$

Finally, the accumulation condition given by the third of Equations (54) on $Y = \eta(\xi)$ gives

$$\frac{d}{d\xi} \left\{ \pm \eta \left[\frac{\mp \eta \eta'}{j \bar{\Lambda}(\eta)} \right]^m - \frac{\eta}{\eta'} g_1(\mp \eta \eta') \mp \frac{g_2(\mp \eta \eta')}{(\eta')^2} \right\} = Q(\eta) - B(\eta) = Q^*(\eta). \quad (68)$$

6. GENERAL PROPERTIES AND VALIDITY

We can rewrite Equation (68) as a first-order differential equation for $\eta'(\xi) = \gamma_0(\eta)$:

$$\left. \begin{aligned} \gamma_0 \frac{d}{d\eta} \left[\pm \eta \left\{ \frac{\mp \eta \gamma_0}{j \bar{\Lambda}(\eta)} \right\}^m \pm \eta^2 \Omega(\mp \eta \gamma_0) \right] &= Q^*(\eta), \\ \Omega(z) &\simeq \Omega_0 z^n \quad \text{as } z \rightarrow 0; \end{aligned} \right\} \quad (69)$$

$n = 1$ is the finite viscosity case. This follows from the definitions (64) and Equations (60) and (61) for a power law and polynomial law respectively. Explicitly

$$\left. \begin{aligned} \text{from Equation (60):} \quad \Omega &= \frac{3^{\frac{1}{2}(n+1)} k (\mp \eta \gamma_0)^n}{n+2}, \\ \text{from Equation (61):} \quad \Omega &= C_0(\mp \eta \gamma_0) + \frac{2}{3} \theta C_1(\mp \eta \gamma_0)^3 + \frac{2^{\frac{1}{2}}}{7} \theta^2 C_2(\mp \eta \gamma_0)^5. \end{aligned} \right\} \quad (70)$$

Now $\int_0^{\epsilon L} Q^* d\xi = 0$ for zero mass flux, and the term in braces in Equation (69) is zero at

$\gamma_0 = 0$ ($\xi = 0$), so must also vanish at $\eta = 0$ ($\xi = \xi_m = \epsilon L$) if Equation (69) holds over the entire range. This requires

$$\left. \begin{aligned} \gamma_0(\eta) &\simeq \mp \gamma_m \eta^\beta (1 - \gamma_1 \eta + \dots), & \beta &\geq 0, \\ \bar{\Lambda}(\eta) &\simeq \lambda_0 \eta^t (1 - \lambda_1 \eta + \dots), & 0 &\leq mt < 1 + m(1 + \beta), \end{aligned} \right\} \quad (71)$$

as $\eta \rightarrow 0$.

As $\eta \rightarrow \eta_c = \eta(0)$, $\bar{\Lambda}(\eta)$ and $Q^*(\eta)$ are finite and $\gamma_0 \rightarrow 0$, so the essential behaviour of Equation (69) is

$$\gamma_0 \frac{d}{d\eta} (\gamma_0^t) = O(1), \quad (72)$$

where $l = \min(m, n), \geq 1$. Hence

$$\gamma_0 \simeq (\eta_c - \eta)^{1/(l+1)} \quad \text{as } \eta \rightarrow \eta_c. \tag{73}$$

As $\eta \rightarrow 0$ ($0 \leq Y \leq \eta$) or $\gamma_0 \rightarrow 0$, the dominant terms of ψ_0 , from Equation (65), are, in both limits,

$$\psi_0 - \int_0^\xi B(\eta) d\xi \simeq Y^{2\eta} \gamma_0^n \text{ or } Y \left(\frac{\eta \gamma_0}{\Lambda(\eta)} \right)^m. \tag{74}$$

Thus, as $\gamma_0 \rightarrow 0, \eta \rightarrow \eta_c$,

$$\psi_0 \simeq (\eta_c - \eta)^{l/(l+1)}. \tag{75}$$

Only derivatives $\partial\psi_0/\partial\xi$ and $\partial^r\psi_0/\partial Y^r$ ($r = 1, 2, 3$) occur in the leading-order equations, but $\partial^2\psi_0/\partial\xi^2, \partial^2\psi_0/\partial\xi \partial Y$ enter coefficients of higher-order terms in the stress expansions. All are bounded as $\eta \rightarrow \eta_c$ for $l \geq 1$. Anticipating, however, that $\partial^3\psi_0/\partial\xi^3$ which arises in higher-order terms of the stress gradient must also be bounded, it is necessary that $l \leq 1$, and hence $|d\gamma_0/d\xi| \leq O(1)$ which implies Weertman's (1961) postulate of slowly varying slope. Direct differentiation of both terms of Equation (74) shows that bounded $\partial^3\psi_0/\partial\xi^3$ requires (i) if $l = n = 1$ then $m = 1$ or 2 or $m \geq 3$; (ii) if $l = m = 1$ then $n = 1$ or 2 or $n \geq 3$. It remains to determine the behaviour of γ_0 as $\eta \rightarrow 0$, and in particular to find any restrictions on m, n, β , and t to ensure that γ_0 and appropriate ψ_0 derivatives are bounded.

Let

$$Q^*(\eta) = -Q_0(1 - q_1\eta + \dots) \quad \text{as } \eta \rightarrow 0, Q_0 > 0, \tag{76}$$

which represents positive ablation plus drainage at the margin. If the first term within the braces in Equation (69) is dominant, then since $1 + m(1 + \beta - t) > 0$ by Equation (71), the lowest power of η on the left-hand side of Equation (69) is $\beta + m(1 + \beta - t), = 0$ to balance the right-hand side; hence

$$\beta = \frac{m(t-1)}{1+m}. \tag{77}$$

The inequalities of (71) require

$$1 \leq t < 2 + \frac{1}{m}, \quad 0 \leq \beta < 1, \tag{78}$$

and $t = 1 \Leftrightarrow \beta = 0$. If the second term within the braces of Equation (69) is dominant, then $\beta = -1$, which is not an acceptable solution. Now the first ξ derivative of the second term of Equation (74), applying Equation (77), $\simeq \eta^{-1}$ if $0 < \beta < 1$, and hence we require $\beta = 0, t = 1$, so that γ_0 is analytic in η and third derivatives of both terms of ψ_0 are bounded with the previous restrictions on m, n . The power of η in the left-hand side of Equation (69) resulting from the second term is now $(n+1) \geq 2$, confirming that only the first term contributes to η^0 and η^1 . An explicit expansion shows that

$$\gamma_m = [Q_0(j\lambda_0)^m]^{1/(1+m)}, \quad \gamma_1 = \frac{q_1 + 2m\lambda_1}{1 + 2m}. \tag{79}$$

Consider the solution of Equation (69) for $-L/\epsilon \leq \xi \leq 0$ with $Q^*(\eta)$ such that the profile is monotonic: $\eta'(\xi) \geq 0, \gamma_0(\eta) \geq 0$. The end condition given by the third of Equations (53) is

$$\gamma_0 = 0 \quad \text{at } \eta = \eta_c, \tag{80}$$

but η_c is a property of the solution, being the height at which the net mass flux becomes zero:

$$\int_0^{\eta_c} \frac{Q^*(\eta) d\eta}{\gamma_0(\eta)} = 0. \tag{81}$$

Here we suppose Q^* is negative near the margin, and increases monotonically with η into positive accumulation, and $\gamma_0 > 0$ for $0 < \eta < \eta_c$. However, Equation (81) is guaranteed for a valid solution under the restrictions (71), and Equation (79) prescribes the behaviour of γ_0 at $\eta = 0$. Thus, we integrate Equation (69) from $\eta = 0$ and determine η_c explicitly as the height at which γ_0 first becomes zero. The profile is determined by evaluating

$$\xi(\eta) = - \int_{\eta}^{\eta_c} \frac{d\eta'}{\gamma_0(\eta')}, \quad \epsilon L = -\xi(0) = \xi_m, \tag{82}$$

bounded as $\eta \rightarrow \eta_c$ in view of Equation (73). In practice, the slope and profile are determined by numerical integration of the simultaneous differential equations

$$\left. \begin{aligned} \frac{d\eta}{d\gamma_0} &= F(\eta, \gamma_0), & \eta(\gamma_m) &= 0, \\ \xi &= \xi + \epsilon L, & \frac{d\xi}{d\gamma_0} &= \gamma_0^{-1} F(\eta, \gamma_0), & \xi(\gamma_m) &= 0, \end{aligned} \right\} \tag{83}$$

obtained from Equations (69) and (82) by using γ_0 as independent variable. Starting at the margin we determine $\eta = \eta_c$ and $\xi = \xi_m = \epsilon L$ when $\gamma_0 = 0$. Since $\beta = 0$, $F(\eta, \gamma_0)$ is defined explicitly at $\eta = 0$, $\gamma_0 = \gamma_m$. Also $\lambda = O(h)$ as $h \rightarrow 0$ which is equivalent to λ being linear in the overburden pressure in view of Equation (55). The combination of linear dependence on height (pressure) and linear dependence on velocity ($m = 1$) was proposed by Bodvarsson (1955). So validity requires a ‘‘perfect slip’’ limit at the margin. Global perfect slip $\bar{\Lambda} = 0$ leads to $\gamma_0 \equiv 0$ which is a uniform-depth solution with no margin. However, this may also be the leading-order solution for non-zero $j\bar{\Lambda} \leq O(\epsilon)$ which is valid in some central zone, and then the margin solution appears to have slope $< O(\epsilon)$. In practice a slope of order ϵ (or greater) at the margin is expected, so $j\bar{\Lambda} = O(1)$ is a reasonable assumption. In the non-slip limit $\bar{\Lambda} \rightarrow \infty$, Equation (69) gives $\beta = -1$, implying that a margin solution with slope greater than order ϵ (not necessarily unbounded) is needed. A new matched asymptotic expansion analysis is required to investigate this situation.

A Newtonian fluid solution is obtained by setting $n = 1$ in Equation (70) when $\Omega = \mp k\eta\gamma_0$. Then, with a linear sliding law ($m = 1$), (69) becomes (for $\xi \leq 0$)

$$\gamma_0 \frac{d}{d\eta} [f(\eta)\gamma_0] = -Q^*(\eta), \quad f(\eta) = k\eta^3 + \frac{\eta^2}{j\bar{\Lambda}(\eta)}, \tag{84}$$

which has the explicit solution

$$\gamma_0(\eta) = \frac{1}{f(\eta)} \left[-2 \int_0^{\eta} f(\eta') Q^*(\eta') d\eta' \right]^{\frac{1}{2}}, \tag{85}$$

since $f(0) = 0$, and Q^* is negative at $\eta = 0$ and increases with η . Note that $f(\eta) \simeq \eta$ as $\eta \rightarrow 0$, since $t = 1$, so γ_0 is finite. Thus $\gamma_0(\eta)$ and $\xi(\eta)$ are determined by simple quadratures, and this evaluation was used to test the accuracy of the numerical integration of the differential equations (83).

Nye’s (1959) profile is deduced from constant accumulation q and constant λ . If we take $q = q_m$ and $\lambda = \lambda(n_0)$, it is

$$\eta^{2+1/m} + \frac{j\bar{\Lambda}(1)(2m+1)}{m+1} \xi^{1+1/m} = \eta_c^{2+1/m}, \tag{86}$$

independent of the ice law, and η_c unknown. Nye proposes $m = \frac{1}{2}(n+1)$ where n is the power in Glen’s law, then

$$\left. \begin{aligned} \eta^{2(n+2)/(n+1)} + \frac{2(n+2)j\bar{\Lambda}(1)}{n+3} \xi^{(n+3)/(n+1)} &= \eta e^{2(n+2)/(n+1)}. \\ \xi_m = \left[\frac{n+3}{2(n+2)j\bar{\Lambda}(1)} \right]^{(n+1)/(n+3)} \eta e^{2(n+2)/(n+3)}, \quad j &= \theta^{(n-1)(n+3)/2(n+1)^2}. \end{aligned} \right\} (87)$$

By choosing η_c in Equation (87) as the central height determined by the preceding solution for various cases, we can compare the profiles and ξ_m of Equation (87) with those of our leading-order solution. However, following Nye's approximate procedure with general $q(h)$, $\lambda(h)$ leads to the differential equation (69) with $\Omega \equiv 0$; that is, the influence of the constitutive response is absent. It is clear from Equations (70) that when η and γ_0 are not much smaller than unity, that is, away from the margin and centre, and $j\bar{\Lambda}(\eta)$ is not much smaller than unity, then the Ω term has the same magnitude as the sliding term, so omission of the Ω contribution is not justified.

7. ILLUSTRATIONS

For comparison between laws with different exponent n , we express all results in terms of the same dimensionless horizontal coordinate based on the scale factor $\epsilon_1 = \epsilon(n = 1)$; that is, using Equations (45),

$$\xi^* = \frac{\epsilon_1}{\epsilon} \xi = \theta^{(n-1)/2(n+1)} \xi. \tag{88}$$

All calculations adopt the values (15) with $a = 0.1$, $q_m = 3 \times 10^{-9} \text{ m s}^{-1}$, so that $\theta = 0.09$ and the real slope magnitude ϵ for different n and h_0 is shown in Table I. In $\xi < 0$ the velocities are given by

$$\left. \begin{aligned} \epsilon_1(U - U_b) &= \theta^{(n-1)/2(n+1)} \{g_1[\gamma_0(\eta - Y)] - g_1[\gamma_0\eta]\} / \gamma_0, \\ \epsilon_1 U_b &= -\theta^{(n-1)/2(n+1)} (\eta \gamma_0 / j\bar{\Lambda})^m, \\ \frac{U_s - U_b}{U_b} &= g_1(\gamma_0\eta) (j\bar{\Lambda} / \gamma_0\eta)^m / \gamma_0, \end{aligned} \right\} (89)$$

where the subscripts b, s denote base and surface values, and

$$\begin{aligned} V + B(\eta) &= Y \gamma_0^{m+1} \frac{d}{d\eta} \{(\eta / j\bar{\Lambda})^m + Y \gamma_0 g[\gamma_0\eta] + g_1[\gamma_0(\eta - Y)] - g_1[\gamma_0\eta]\} + \\ &+ \frac{d\gamma_0}{d\eta} \left[m Y (\gamma_0\eta / j\bar{\Lambda})^m + Y \eta g[\gamma_0\eta] + \frac{\eta - Y}{\gamma_0} g_1[\gamma_0(\eta - Y)] - \right. \\ &\quad \left. - \frac{\eta + Y}{\gamma_0} g_1[\gamma_0\eta] - \frac{2}{\gamma_0^2} (g_2[\gamma_0(\eta - Y)] - g_2[\gamma_0\eta]) \right]. \end{aligned} \tag{90}$$

Both linear and exponential forms of the accumulation–drainage function $Q^*(\eta)$ and the sliding coefficient $\bar{\Lambda}(\eta)$ have been treated, namely

$$Q^*(\eta) = -Q_0 + (1 + Q_0) \eta \text{ or } -Q_0 + (1 + Q_0)(1 - \exp(-s\eta)), \tag{91}$$

$$\bar{\Lambda}(\eta) = \lambda_0 \eta \text{ or } \lambda_0(1 - \exp(-r\eta)) / r. \tag{92}$$

The exponential forms with large s and r imply small gradients $Q^{*'}$ and $\bar{\Lambda}'$ for η order unity, which correspond better with Nye's constant Q^* , $\bar{\Lambda}$ approximations than the linear functions. A case $s = r = 5$ is shown in Figure 2 together with an example using the linear forms, both with $\lambda_0 = Q_0 = 1$, comparing Nye's profiles given by Equation (87) with the corresponding

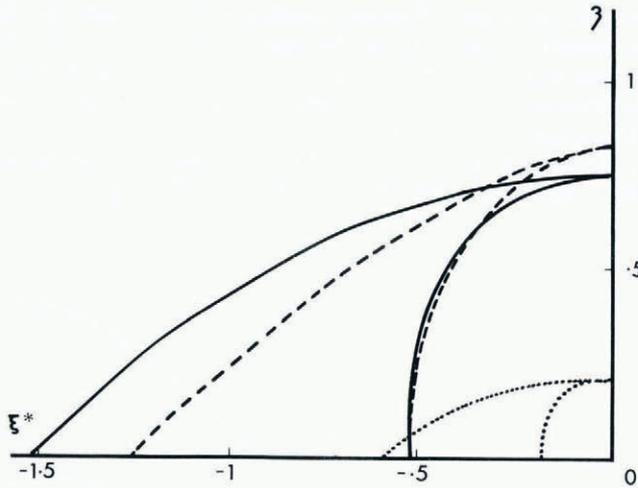


Fig. 2. Comparison of small-slope profile for Glen's law with Nye's profile for $\lambda_0 = Q_0 = 1$.
 (i) $m = 1, n = 1$, and exponential λ and Q^* , $s = r = 5$.
 (ii) ——— $m = 1, n = 1$, and linear λ and Q^* .

complete small-slope solution. As demonstrated earlier, the constitutive response term Ω in Equation (6g) cannot be neglected, but the examples in Figure 2 illustrate the entirely different profiles obtained with the further assumptions of constant Q^* and $\bar{\Lambda}$.

Next we indicate how the choice of constitutive law between the Colbeck and Evans polynomial law and Glen's power law with $n = 1$ and $n = 3$ affects the solution. Table II shows values of the semi-length ξ_m^* and height η_c for the linear forms of Equations (91) and (92) with $m = 1$, and λ_0, Q_0 between 1 and 10. The margin distance and centre height represent the maximum profile discrepancies. The greatest differences arise in ξ_m^* , increasing with increasing λ_0 for given law and given Q_0 . As $\bar{\Lambda}$ increases, the constitutive term has more influence, and the maximum differences in ξ_m^* occur for $\lambda_0 = 10$ with changes of 18 to 22% for the different Q_0 between CE and $n = 3$. For moderate λ_0 the discrepancies are small, and the following illustrations are all based on the polynomial law, which eliminates the parameter n , and use the linear $Q^*(\eta), \bar{\Lambda}(\eta)$.

Figure 3 shows profiles for four sets of (λ_0, Q_0, m) . Increasing Q_0 with λ_0 and m fixed decreases ξ_m^* and increases η_c ; that is, large ablation at the margin decreases the length as expected. Increasing λ_0 with Q_0 and m fixed decreases ξ_m^* significantly but makes little

TABLE II. SEMI-LENGTH ξ_m^* , HEIGHT η_c , FOR LINEAR ACCUMULATION $Q^*(\eta)$ AND LINEAR SLIDING LAW ($m = 1$) WITH LINEAR COEFFICIENT $\bar{\Lambda}(\eta)$. SOLUTIONS FOR COLBECK AND EVANS LAW (CE) AND GLEN'S LAW WITH $n = 1, 3$

Q_0	λ_0 Law	1		5		10	
		ξ_m^*	η_c	ξ_m^*	η_c	ξ_m^*	η_c
1	CE	1.542	0.742	0.758	0.719	0.590	0.702
	1	1.534	0.743	0.741	0.724	0.568	0.708
	3	1.500	0.750	0.675	0.749	0.484	0.747
5	CE	1.207	1.216	0.683	1.156	0.587	1.128
	1	1.186	1.222	0.632	1.165	0.520	1.132
	3	1.128	1.248	0.564	1.223	0.458	1.203
10	CE	0.950	1.321	0.572	1.255	0.507	1.230
	1	0.924	1.329	0.504	1.262	0.420	1.226
	3	0.882	1.357	0.480	1.318	0.410	1.296

difference to η_c ; that is, a larger "friction coefficient" decreases the horizontal spread. Similarly, increasing m with λ_0 and Q_0 fixed decreases ξ_m^* . Figure 4 shows the basal velocity distributions for the same sets, all approximately linear on the normalized scale. The margin velocity is noticeably greater for the largest ablation, but the gradient along the base is not strongly influenced by change of λ_0 or m .

Finally, Figure 5 shows both horizontal and vertical velocity distributions with height at various distances from the centre, for the case $\lambda_0 = 1$, $Q_0 = 5$, $m = 1$. $\partial V/\partial X$ is nearly

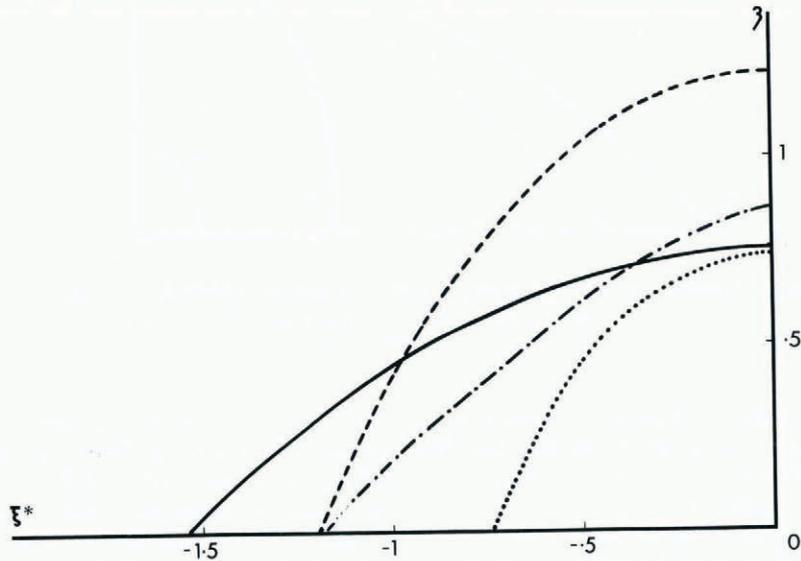


Fig. 3. Comparison of profile for Colbeck and Evans law with linear λ and Q^* .

- (i) ——— $\lambda_0 = 1, Q_0 = 1, m = 1.$
- (ii) - - - - $\lambda_0 = 1, Q_0 = 5, m = 1.$
- (iii) $\lambda_0 = 5, Q_0 = 1, m = 1.$
- (iv) - . . . - $\lambda_0 = 1, Q_0 = 1, m = 3.$

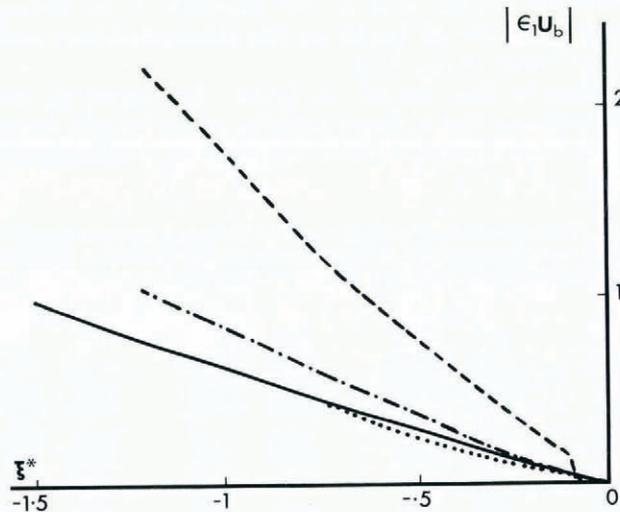


Fig. 4. Basal velocities $|\epsilon_1 U_b(\xi^*)|$ corresponding to Figure 3 profiles.

constant in both \mathcal{Y} and ξ^* , implying the same for $\partial U/\partial X$ by mass balance, and demonstrating that $\partial V/\partial \mathcal{Y}$ and $\partial U/\partial X$ are both of order unity. While $\epsilon_1 \partial U/\partial \mathcal{Y}$ is small compared with unity except, perhaps, near the bed, it does not follow that $\partial U/\partial \mathcal{Y}$ is small, and though the basal sliding velocity is much greater than the differential velocity between base and surface as noted by Nye (1959), the contribution of $\partial U/\partial \mathcal{Y}$ to the shear strain-rate, and hence the Ω

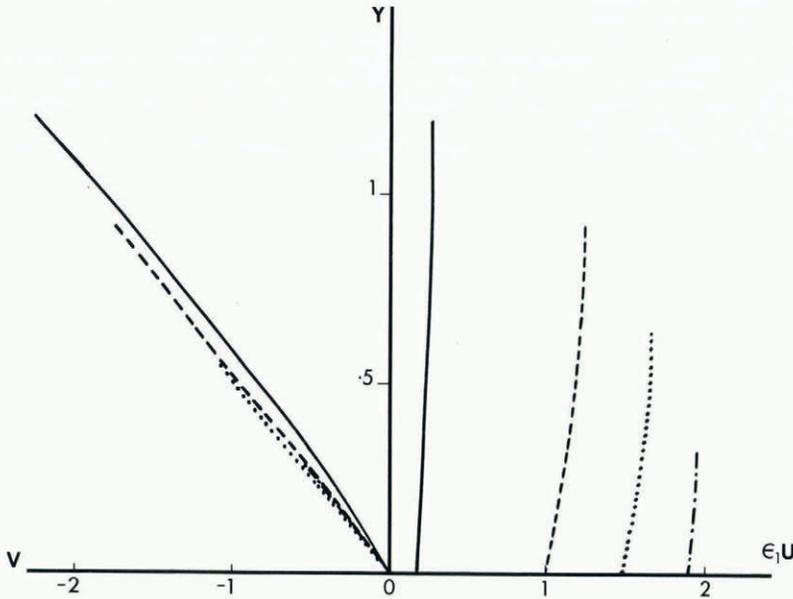


Fig. 5. Horizontal and vertical velocity distribution with height for linear λ and Q^* with $\lambda_0 = 1$, $Q_0 = 5$, $m = 1$, at positions

- (i) $\xi^* = -0.13$ ———
- (ii) $\xi^* = -0.63$ - - - - -
- (iii) $\xi^* = -0.87$ ·····
- (iv) $\xi^* = -1.04$ - · - · - ·

term in Equation (69), is not negligible. Since $\partial V/\partial \xi^*$ is order unity, the contribution $\partial V/\partial X$ is $O(\epsilon_1)$, so $\partial U/\partial \mathcal{Y}$ does indeed make a significant contribution. This highlights the need for a rational scaling of coordinates and physical variables to obtain the correct gradient balances in the field equation, and such scaling should be dictated by the boundary conditions or driving terms for the flow. Clearly our approach is appropriate to a variety of ice-sheet flow problems in which the driving mechanism suggests small-slope features.

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REFERENCES

- Bodvarsson, G. 1955. On the flow of ice-sheets and glaciers. *Jökull*, År 5, p. 1-8.
- Colbeck, S. C., and Evans, R. J. 1973. A flow law for temperate glacier ice. *Journal of Glaciology*, Vol. 12, No. 64, p. 71-86.
- Glen, J. W. 1958. The flow law of ice: a discussion of the assumptions made in glacier theory, their experimental foundations and consequences. *Union Géodésique et Géophysique Internationale. Association Internationale d'Hydrologie Scientifique. Symposium de Chamonix, 16-24 sept. 1958*, p. 171-83. (Publication No. 47 de l'Association Internationale d'Hydrologie Scientifique.)
- Morland, L. W. 1979. Constitutive laws for ice. *Cold Regions Science and Technology*, Vol. 1, No. 2, p. 101-08.
- Nye, J. F. 1952. A method of calculating the thicknesses of the ice-sheets. *Nature*, Vol. 169, No. 4300, p. 529-30.
- Nye, J. F. 1959. The motion of ice sheets and glaciers. *Journal of Glaciology*, Vol. 3, No. 26, p. 493-507.
- Paterson, W. S. B. 1969. *The physics of glaciers*. Oxford, etc., Pergamon Press. (The Commonwealth and International Library. Geophysics Division.)
- Weertman, J. 1961. Equilibrium profile of ice caps. *Journal of Glaciology*, Vol. 3, No. 30, p. 953-64.